#### MOISTURE & DENSITY TEST ISI Lab No.: G-52574 Client: URS/ARUP/HMM JV Project : California High Speed Train Job no: 2636-001.0 Boring # S0067R S0067R S0067R S0067R S0067R S0067R Sample # MC04-2 MC12-2 SS19 MC26-2 MC28-2 SS31 Depth (ft.) 15.5-16 50.5-51 81-81.5 110.5-111 120.9-121.4 136-136.5 Soil type: (visual) Olive brown sandy Greenish gray Dark grayish Greenish gray Dark greenish Dark greenish silty clay with sand green clay with clay silty sand gray sandy clay gray lean clay with sand sand Date tested: 10/31/13 10/31/13 10/28/13 10/31/13 10/31/13 10/28/13 JH JH 2 Tested by: JH JH JH JH 2. 3. Specimen height (in.) 6.00 5.80 6.00 5.31 3. 4. Wt. of specimen + tare (gm) 894.96 882.63 874.24 828.76 0.00 0.00 0.00 0.00 Tare wt. (gm) Diameter (in.) 2.40 2.39 2.41 2.41 7. Wet wt. of soil + dish wt. (gm) 256.55 226.38 167.53 284.16 229.08 88.91 Dry wt. of soil + dish wt. (gm) 211.68 192.80 144.55 232.20 200.11 77.91 8. Wt. of dish (gm) 51.01 50.41 50.41 51.06 50.62 30.38 9. 10. Dish ID 10. Wet Density ( pcf ) 125.5 129.1 121.6 130.2 Dry Density ( pcf ) 98.1 104.5 94.5 109.1 Moisture Content (%) 27.9 28.7 23.6 24.4 19.4 23.1 Gs (Assumed) 2.70 2.70 2.70 2.70 2.70 2.70 2.70 2.70 Void Ratio 0.717 0.613 0.783 0.544 Saturation (%) 105.1 103.9 98.9 96.1 Additional data: Wt. of dry soil + dish before washing (gm) Wt. of dry soil + dish after washing (gm) % Passing # 200 sieve

**USCS** symbol



### **Assignment Sheet / Density Test**

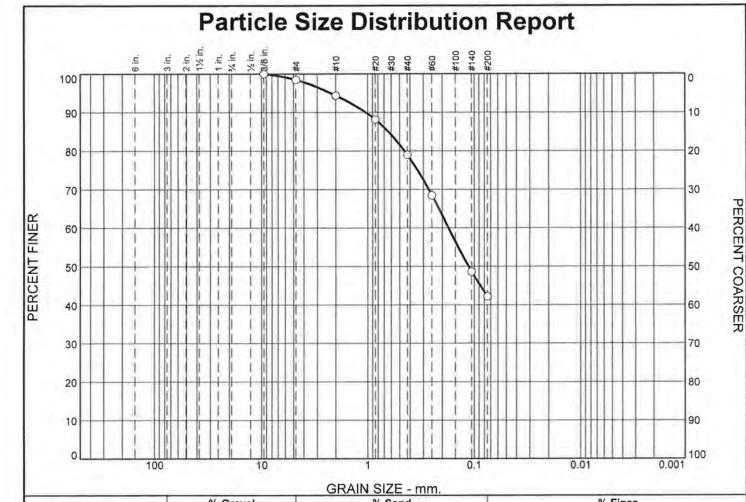
Project Number : 23502-ZS9 Lab. Tech : K. Ford Project Name : HSR Date Completed : 1/20/14

Date Drilled : 1/8/14

Boring	Sample	Depth	Tests	Soil Wt	Length	Diameter	Wet Wt	Dry Wt	Wet	Moisture	Dry	Soil
				Gms	in	in	Gms	Gms	Density	%	Density	Classification
:0019AR	MC03-2	15.5-16'	SA				200	174.1		14.9%		SP
S0019AR	SS06	30-31.5'	SA				200	181.5		10.2%		SM/SP
S0020R	SS07	25-26.5'	SA				200	167.7		19.3%		SM
S0021R	MC10-1	46-46.5'	SA				200	180.2		11.0%		SM/ML
S0021R	MC18-1	86-86.5'	SA				200	184.8		8.2%		SP
S0021R	SS07	30-31.5'	SA				200	171.3		16.8%		SM/SP
S0029R	MC08-1	30.9-31.4'	SA				200	174.1		14.9%		SM
S0031R	MC03-2	10.4-11	SA				200	166.5		20.1%		SP
S0031R	SS08	35-36.5'	SA				200	175.4		14.0%		SM
S0034BR	MC09-1	41-41.5'	HY,SA									SM/ML
S0065R	MC04-2	15.5-16'	SA				200	172.6		15.9%		SM/SP
S0066R	MC03-2	10-11.5'	SA				200	172.0		16.3%		SM
S0067R	MC06-1	25-26.5'	SA				200	169.4		18.1%		SP
S0067R	MC11-1	45-46.5'	HY,SA									SM
S0067R	MC23-1	95-96.4'	HY,SA									SM
S0070R	MC09-2	40.5-41'	HY,SA									SM
S0070R	U05	20-22'	HY,SA									SM
S0072R	MC12-1	51-51.5'	HY,SA									SM
S0073R	MC11-2	45.5-16'	HY,SA									ML/CL
	_		7 -									

Notes:

CHEM Sulfate/Chloride MR Minimum Resistivity Collapse РΗ COLL pH Test CONSOL 1D Consolidation Ы Atterberg Limits CURV Modified Proctor RV R-value DD Moisture Density RVT R-value Treated DS Direct Shear SA Sieve Analysis HY Hydrometer TRX Triaxial Compression



% +3"		% Gr	avel		% Sand		%	Fines	
% +3	(	Coarse	Fine	Coarse	e Medium Fine Silt		Clay		
0		0	1	5	15	37	42		
SIEVE	PERCENT	SPE	c.*	PASS?		- 6	Soil Description		
SIZE	FINER	PERC	ENT	(X=NO)	Olive clayey sand				
3/8	100								

1	SIEVE	FINER	SPEC." PERCENT	PASS? (X=NO)
-	SIZE		PERCENT	(V-NO)
-91	3/8	100		
П	#4	99		
-1	#10	94		
-1	#20	88		
- 1	#40	79		
	#60	68		
-1	#140	49		
- 1	#200	42		
П		1 1 1		
- 1				
П				
- 1				
П				

Soil Description	
d	
Atterberg Limits	PI=
Coefficients D <sub>85</sub> = 0.6400 D <sub>30</sub> = C <sub>u</sub> =	D <sub>60</sub> = 0.1749 D <sub>15</sub> = C <sub>c</sub> =
Classification AASHT	0=
Remarks	
	Atterberg Limits LL= Coefficients D85= 0.6400 D30= Cu= Classification AASHTO

(no specification provided)

Source of Sample: S0067R G-52574 Sample Number: U03

Depth: 11.5-12.5

Client: URS/ARUP/HMM JV Project: California High Speed Train

Project No: 2636-001.0

**Figure** 





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### Sieve Analysis for Soil / Fine Aggregate ASTM C-136

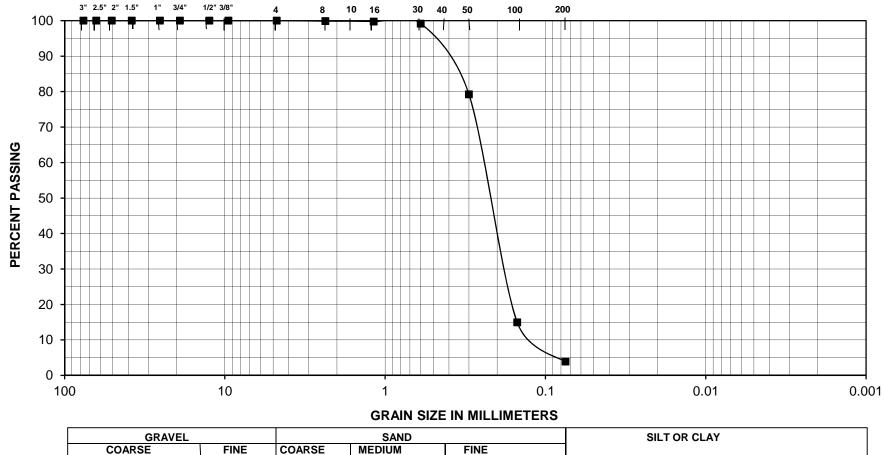
Project:	CA HSR		Technician:	K. Ford		
			Date:	1/16/2014		
TES#:	23502-ZS9		Sample No.:	MC06-1		
Boring #:	S0067R; 25-26.5		Classification:	(SP) Poorly Graded Sand		
	•					
		Weight	Maximum	Minimum V	Veight of	
		(lbs. or grams)	Sieve Size	Test Specimen, lbs. (kg)		
Total Dry S	Sample + Tare Wt.		Sand	1.0 (0	0.5)	
Tare Weig	ht		3/8"	2.0 (1	.0)	
Total Dry S	Sample Wt.	169.4	1/2"	4.0 (2	2.0)	
Initial Weig	ght Fine		3/4"	11.0 (	5.0)	
Aggregate	Before Wash	169.4	1"	22.0 (1	0.0)	
Final Weig	ht Fine		1 1/2"	33.0 (1	5.0)	
Aggregate After Wash		164.8	2"	44.0 (2	(0.0)	
	Cumulative	Individual	Cumulative	Cumulative		
Ciava	14/0:0/04	\//a:abta	0/	0/	1	

	Cumulative	Individual	Cumulative	Cumulative	
Sieve	Weight	Weights	%	%	
Size	Retained	Retained	Retained	Passing	Specs.
3 in.			0.0	100.0	
2 1/2 in.			0.0	100.0	
2 in.			0.0	100.0	
1 1/2 in.			0.0	100.0	
1 in.			0.0	100.0	
3/4 in.			0.0	100.0	
1/2 in.			0.0	100.0	
3/8 in.			0.0	100.0	
#4	0.0	0.0	0.0	100.0	
#8	0.2	0.2	0.1	99.9	
#16	0.4	0.2	0.2	99.8	
#30	1.4	1.0	0.8	99.2	
#50	35.2	33.8	20.8	79.2	
#100	144.1	108.9	85.1	14.9	
#200	162.8	18.7	96.1	3.9	
Pan	164.8				



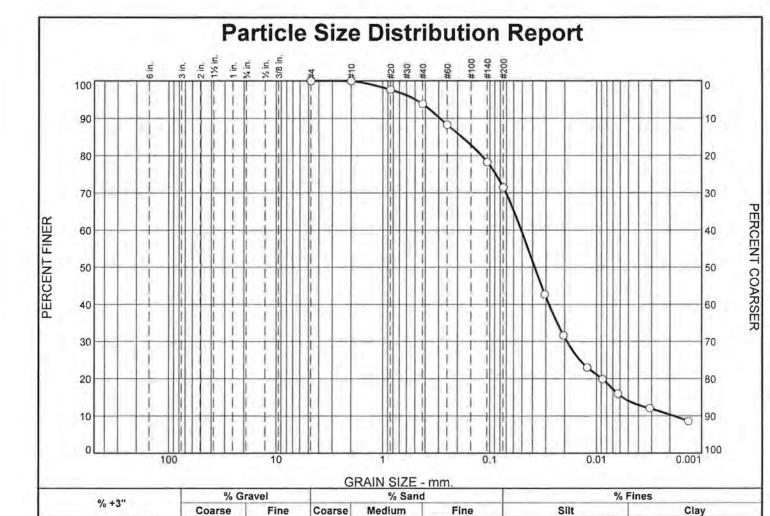
#### U.S. STANDARD SIEVE OPENING IN INCHES

#### **U.S. STANDARD SIEVE NUMBERS**



—**■** MC06-1

Sample #	Classification	% Gravel	% Sand	% Silt	% Clay	% Moist.	LL	PL	PI	Project:	CA HSR
MC06-1	(SP) Poorly Graded Sand	0	96.1	3.9							
										TES#:	23502-ZS9
										Boring #	S0067R; 25-26.5'
										Date:	1/16/2014



SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
	77 (75 76 85 7	PERCENT	(X-140)
#4	100		
#10	100		
#20	98		
#40	94		
#60	88		
#140	78		
#200	71		
0.0308 mm.	43		
0.0205 mm.	32		
0.0123 mm.	23		
0.0088 mm.	20		
0,0063 mm.	16		
0.0031 mm.	12		
0.0013 mm.	9		

0

0

0

Gray	y brown sand	Soil Description ly silt	
PL=		Atterberg Limits	PI=
D90 D50 D10	0.2924 = 0.0384 = 0.0018	Coefficients D <sub>85</sub> = 0.1826 D <sub>30</sub> = 0.0190 C <sub>u</sub> = 27.86	D <sub>60</sub> = 0.0512 D <sub>15</sub> = 0.0056 C <sub>c</sub> = 3.84
USC	CS=	Classification AASHTO	)=
F.M	.=0.32	Remarks	

(no specification provided)

0

Source of Sample: S0067R G-52574 Sample Number: SS09

Depth: 41.0-41.5

Date: 10/31/13



Client: URS/ARUP/HMM JV

Project: California High Speed Train

Project No: 2636-001.0

**Figure** 

Tested By: JH/PH

Checked By: PH



Construction Testing & Inspection \* Geotechnical & Environmental Engineering

## Sieve Analysis for Soil and Fine Aggregate

 Project:
 CA HSR FRE\_BAK
 Technician:
 K. Ford

 TES#:
 23502-ZS9
 Date:
 1/14/2014

 Boring No.:
 S0067R
 Depth, ft
 45-46.5'

 Sample No.:
 MC11-1
 Classification:
 (SM) Fine-Med Silty Sand

	Weight (grams)	Maximum Sieve Size	Minimum Weight of Test Specimen, lbs. (kg)
Total Dry Sample + Tare Wt.	,	Sand	1.0 (0.5)
Tare Weight		3/8"	2.0 (1.0)
Total Dry Sample Wt.	74.4	1/2"	4.0 (2.0)
Initial Weight Fine		3/4"	11.0 (5.0)
Soil Before Wash	74.4	1"	22.0 (10.0)
Final Weight Fine		1 1/2"	33.0 (15.0)
Soil After Wash	64.9	2"	44.0 (20.0)

	Individual	Individual	Combined	Combined	
Sieve	Weight	%	%	%	
Size	Retained	Retained	Retained	Passing	Specs.
3 in.	0.0	0.0	0.0	100.0	
2 1/2 in.	0.0	0.0	0.0	100.0	
2 in.	0.0	0.0	0.0	100.0	
1 1/2 in.	0.0	0.0	0.0	100.0	
1 in.	0.0	0.0	0.0	100.0	
3/4 in.	0.0	0.0	0.0	100.0	
1/2 in.	0.0	0.0	0.0	100.0	
3/8 in.	0.0	0.0	0.0	100.0	
#4	0.0	0.0	0.0	100.0	
#8	0.1	0.1	0.1	99.9	
#10	0.0	0.0	0.1	99.9	
#16	0.2	0.3	0.4	97.6	
#30	14.0	18.8	19.2	80.8	
#40	10.5	14.1	33.3	66.7	
#50	15.2	20.4	53.7	46.3	
#100	12.7	17.1	70.8	29.3	
#200	9.6	12.9	83.7	16.4	
Pan					



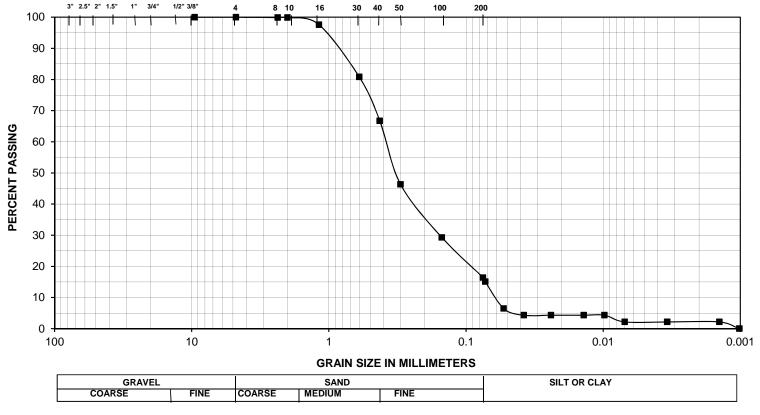
### **HYDROMETER TEST DATA SUMMARY ASTM D 422-63**

PROJECT:		CA HSR F	RE_BAK			TES#:	S0067R
Boring Number		S006	67R	_		DATE:	1/14/2014
Sample Depth	, ft	45-46.5'		Sample No.:	MC11-1	TESTED BY: K. Ford	
					_		
Mass of Test S			75.00	"air-dried"		Hydrometer Type	151H
Mass of Hygro	scopic Sample, g		30.00	"air-dried"			
Mass of Hygro	scopic Sample, g		29.77	"oven-dried"	Specific Gravity of	of Test Material	2.650
Mass of Test S	Sample, g		74.43	"oven-dried"	Specific Gravity of	of Test Solution	Varies
Time	Hydrometer	Corrected	Temperature	Effective Depth	Constant, K	Diameter, D	Amt. Suspended, P
(min.)	Reading	Reading	Degrees C	Table 2 (cm)	Table 3	(mm)	(%)
0.5	1.009	1.007	21	14.4	0.01348	0.0723	15.1
1	1.005	1.003	21	15.5	0.01348	0.0531	6.5
2	1.004	1.002	21	15.8	0.01348	0.0379	4.3
5	1.004	1.002	21	15.8	0.01348	0.0240	4.3
15	1.004	1.002	21	15.8	0.01348	0.0138	4.3
30	1.004	1.002	21	15.8	0.01348	0.0098	4.3
60	1.003	1.001	21	16.0	0.01348	0.0070	2.2
250	1.003	1.001	21	16.0	0.01348	0.0034	2.2
1440	1.003	1.001	21	16.0	0.01348	0.0014	2.2
2880	1.002	1.000	21	16.3	0.01348	0.0010	0.0



#### U.S. STANDARD SIEVE OPENING IN INCHES

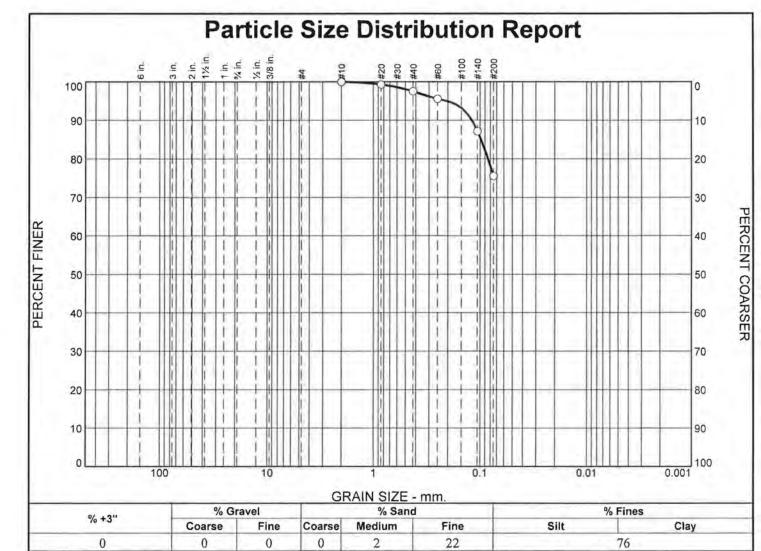
#### U.S. STANDARD SIEVE NUMBERS



45-46.5
---------

Sample #	Classification	% Gravel	% Sand	% Silt	% Clay*	% Moist.	LL	PL	ΡI	Project:	CA HSR FRE_BAK
MC11-1	(SM) Fine-Med Silty Sand	0	83.7	14.2	2.1	0.8					
										TES#:	23502-ZS9
										Boring#:	S0067R
										Date:	1/14/2014

<sup>\*</sup> Particles smaller than 5 Micron in diameter



#10 100 #20 99 #40 98 #60 96 #140 87 #200 76	SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
#40 98 #60 96 #140 87				
#60 96 #140 87				
#140 87		100000000000000000000000000000000000000		
#200 76				

	Soil Description	
Greenish gray si	lty clay with sand	
PL= 22	Atterberg Limits	PI= 10
D <sub>90</sub> = 0.1199 D <sub>50</sub> = D <sub>10</sub> =	Coefficients D85= 0.0982 D30= Cu=	D <sub>60</sub> = D <sub>15</sub> = C <sub>c</sub> =
USCS= CL	Classification AASHT	O= A-4(7)
F.M.=0.12	Remarks	

(no specification provided)

Source of Sample: S0067R G-52574 Sample Number: MC12-2

Depth: 50.5-51.0

Client: URS/ARUP/HMM JV Project: California High Speed Train

Project No: 2636-001.0

**Figure** 





Construction Testing & Inspection \* Geotechnical & Environmental Engineering

## Sieve Analysis for Soil and Fine Aggregate

 Project:
 CA HSR FRE\_BAK
 Technician:
 K. Ford

 TES#:
 23502-ZS9
 Date:
 1/14/2014

 Boring No.:
 S0067R
 Depth, ft
 95-96.4'

 Sample No.:
 MC23-1
 Classification:
 (SM) Fine Silty Sand

	Weight (grams)	Maximum Sieve Size	Minimum Weight of Test Specimen, lbs. (kg)
Total Dry Sample + Tare Wt.	, σ	Sand	1.0 (0.5)
Tare Weight		3/8"	2.0 (1.0)
Total Dry Sample Wt.	74.6	1/2"	4.0 (2.0)
Initial Weight Fine		3/4"	11.0 (5.0)
Soil Before Wash	74.6	1"	22.0 (10.0)
Final Weight Fine		1 1/2"	33.0 (15.0)
Soil After Wash	60.5	2"	44.0 (20.0)

	Individual	Individual	Combined	Combined	
Sieve	Weight	%	%	%	
Size	Retained	Retained	Retained	Passing	Specs.
3 in.	0.0	0.0	0.0	100.0	
2 1/2 in.	0.0	0.0	0.0	100.0	
2 in.	0.0	0.0	0.0	100.0	
1 1/2 in.	0.0	0.0	0.0	100.0	
1 in.	0.0	0.0	0.0	100.0	
3/4 in.	0.0	0.0	0.0	100.0	
1/2 in.	0.0	0.0	0.0	100.0	
3/8 in.	0.0	0.0	0.0	100.0	
#4	0.0	0.0	0.0	100.0	
#8	0.0	0.0	0.0	100.0	
#10	0.0	0.0	0.0	100.0	
#16	0.2	0.3	0.3	99.3	
#30	3.8	5.1	5.4	94.6	
#40	3.2	4.3	9.6	90.4	
#50	5.3	7.1	16.8	83.2	
#100	17.8	23.9	40.6	59.4	
#200	23.7	31.8	72.4	27.6	
Pan					



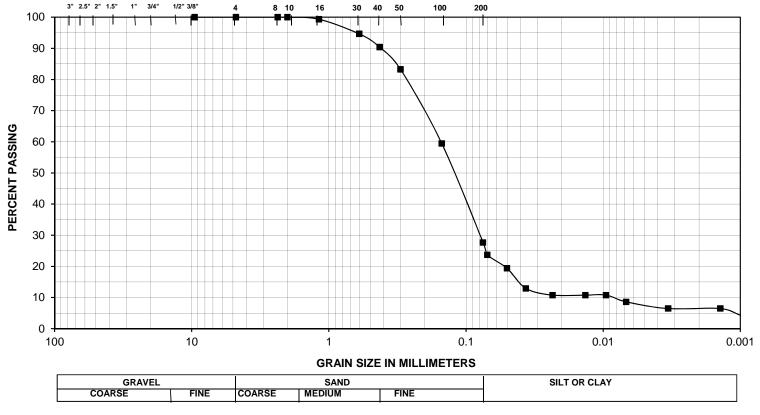
### **HYDROMETER TEST DATA SUMMARY ASTM D 422-63**

PROJECT:		CA HSR F	RE_BAK			TES#:	S0067R
Boring Number		S006	67R	_		DATE:	1/14/2014
Sample Depth	, ft	95-9	6.4'	Sample No.:	MC23-1	TESTED BY:	K. Ford
					<b>-</b>		
Mass of Test S			75.00	"air-dried"		Hydrometer Type	151H
Mass of Hygro	scopic Sample, g		30.00	"air-dried"			
Mass of Hygro	scopic Sample, g		29.85	"oven-dried"	Specific Gravity of	of Test Material	2.650
Mass of Test S	Sample, g		74.63	"oven-dried"	Specific Gravity of	of Test Solution	Varies
Time	Hydrometer	Corrected	Temperature	Effective Depth	Constant, K	Diameter, D	Amt. Suspended, P
(min.)	Reading	Reading	Degrees C	Table 2 (cm)	Table 3	(mm)	(%)
0.5	1.013	1.011	21	13.4	0.01348	0.0698	23.7
1	1.011	1.009	21	13.9	0.01348	0.0503	19.4
2	1.008	1.006	21	14.7	0.01348	0.0365	12.9
5	1.007	1.005	21	15.0	0.01348	0.0233	10.8
15	1.007	1.005	21	15.0	0.01348	0.0135	10.8
30	1.007	1.005	21	15.0	0.01348	0.0095	10.8
60	1.006	1.004	21	15.2	0.01348	0.0068	8.6
250	1.005	1.003	21	15.5	0.01348	0.0034	6.5
1440	1.005	1.003	21	15.5	0.01348	0.0014	6.5
2880	1.004	1.002	21	15.8	0.01348	0.0010	4.3
			<u> </u>				



#### U.S. STANDARD SIEVE OPENING IN INCHES

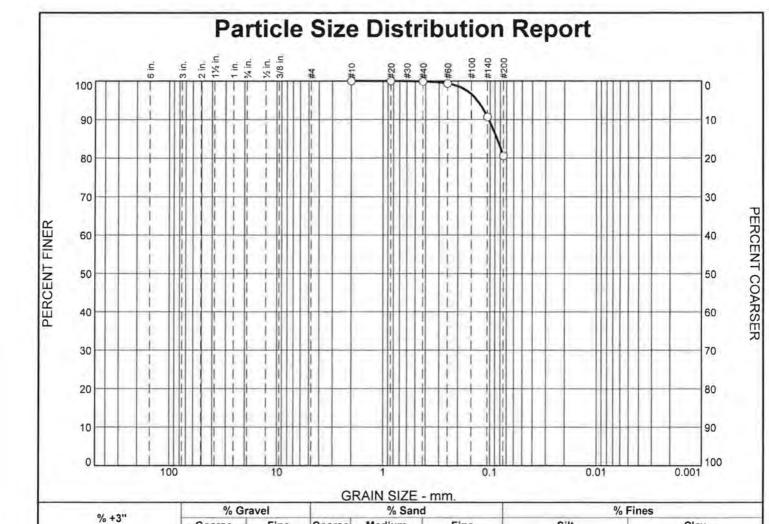
#### U.S. STANDARD SIEVE NUMBERS



_	95-96 //
-	30.4

Sample #	Classification	% Gravel	% Sand	% Silt	% Clay*	% Moist.	LL	PL	PΙ	Project:	CA HSR FRE_BAK
MC23-1	(SM) Fine Silty Sand	0	72.4	20.0	7.7	0.5					
										TES#:	23502-ZS9
										Boring#:	S0067R
										Date:	1/14/2014

<sup>\*</sup> Particles smaller than 5 Micron in diameter



SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
#10	100		()
#20	100		
#40	100		
#60	99		
#140	91		
#200	81		
	1		

Coarse

Fine

0

Coarse

0

Medium

Fine

Dark greenish g	Soil Description ray clayey sand	
PL=	Atterberg Limits	PI=
D <sub>90</sub> = 0.1030 D <sub>50</sub> = D <sub>10</sub> =	Coefficients D <sub>85</sub> = 0.0863 D <sub>30</sub> = C <sub>u</sub> =	D <sub>60</sub> = D <sub>15</sub> = C <sub>c</sub> =
USCS=	Classification AASHT	O=
F.M.=0.04	Remarks	

Silt

Clay

(no specification provided)

Source of Sample: S0067R G-52574 Sample Number: SS24

0

Depth: 101.0-101.5

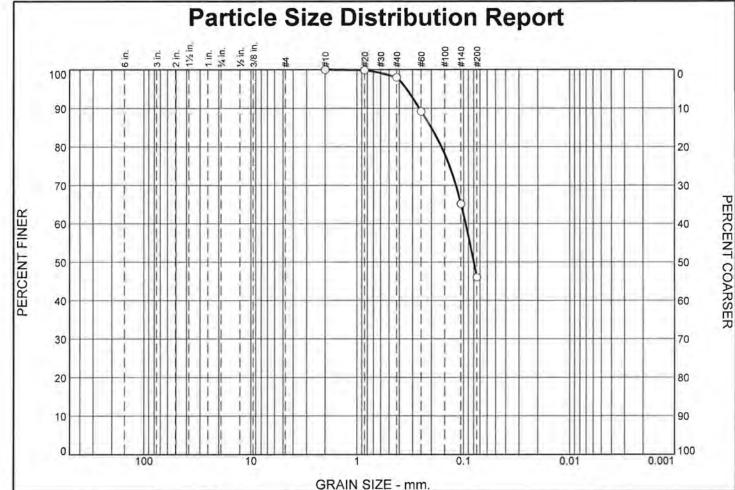
Client: URS/ARUP/HMM JV

Project: California High Speed Train

Project No: 2636-001.0

**Figure** 





% +3"	% Gr	avel	1	% Sand		% Fines		
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay	
0	0	0	0	2	52	46		

SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
#10	100		
#20 #40	100 98		
#60	89		
#140	65		
#200	46		

	Soil Description	
Greenish gray si	lty sand	
		•
PL=	Atterberg Limits LL=	PI=
D <sub>90</sub> = 0.2608 D <sub>50</sub> = 0.0802 D <sub>10</sub> =	Coefficients D <sub>85</sub> = 0.2014 D <sub>30</sub> = C <sub>u</sub> =	D <sub>60</sub> = 0.0957 D <sub>15</sub> = C <sub>c</sub> =
USCS=	Classification AASHTC	=
F.M.=0.30	Remarks	

(no specification provided)

Source of Sample: S0067R G-52574 Sample Number: MC26-2

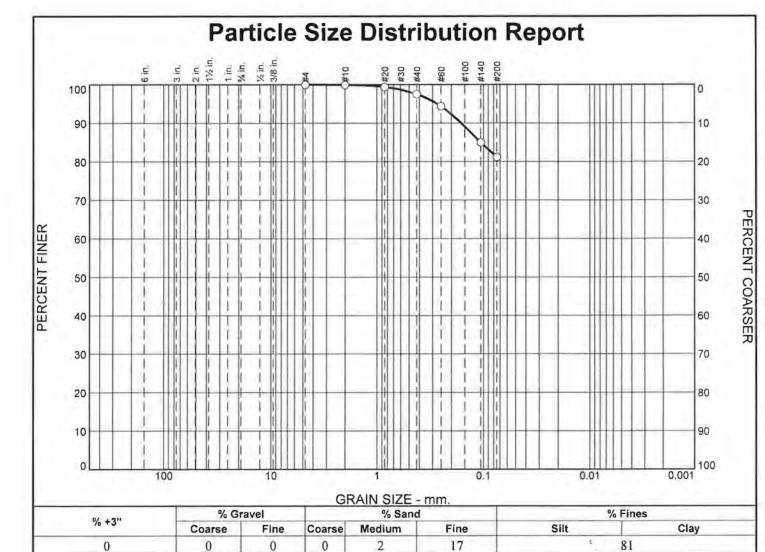
Depth: 110.5-111.0

Client: URS/ARUP/HMM JV Project: California High Speed Train

Project No: 2636-001.0

**Figure** 





#4 100 #10 100 #20 99 #40 98 #60 94 #140 85 #200 81	#4 100 #10 100 #20 99 #40 98 #60 94 #140 85	SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
#20 99 #40 98 #60 94 #140 85	#20 99 #40 98 #60 94 #140 85	#4	100		
#40 98 #60 94 #140 85	#40 98 #60 94 #140 85		100		
#60 94 #140 85	#60 94 #140 85	#20	99		
#140 85	#140 85	#40	98		
25 E 2 E 1 9 S	25 E 2 E 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	#60	94		
#200 81	#200 81	#140	85	)	
		#200	81		

	Soil Description	
Dark greenish g	ray lean clay with san	d
PL= 16	Atterberg Limits	PI= 25
D <sub>90</sub> = 0.1624 D <sub>50</sub> = D <sub>10</sub> =	Coefficients D85= 0.1060 D30= Cu=	D <sub>60</sub> = D <sub>15</sub> = C <sub>c</sub> =
USCS= CL	Classification AASHT	O= A-7-6(19)
F.M.=0.17	Remarks	

(no specification provided)

Source of Sample: S0067R G-52574 Sample Number: SS31

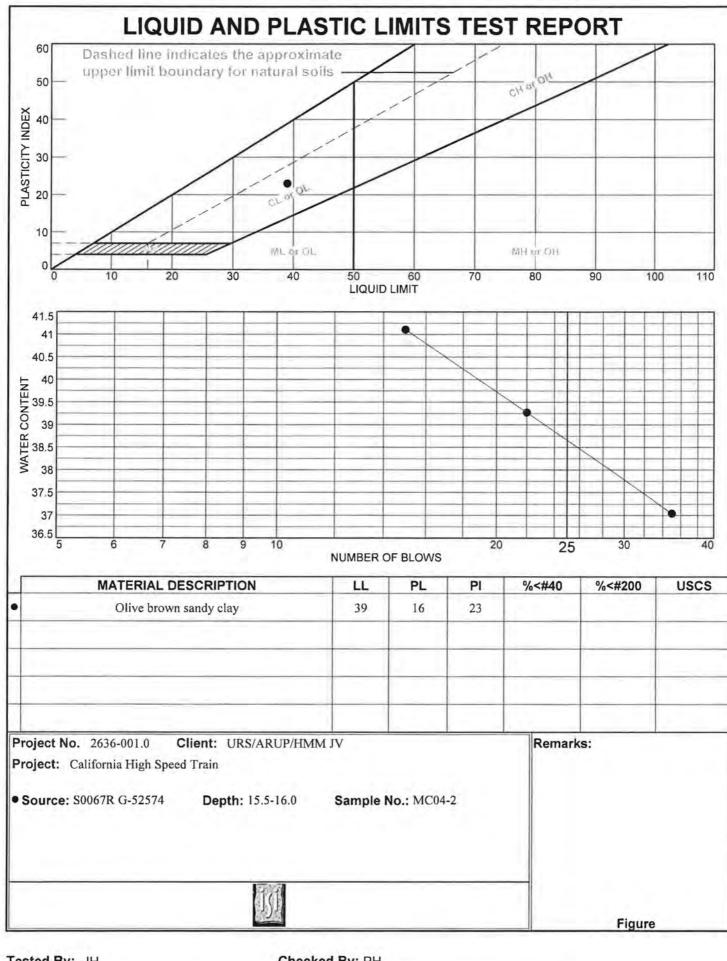
Depth: 136.0-136.5

Client: URS/ARUP/HMM JV Project: California High Speed Train

Project No: 2636-001.0

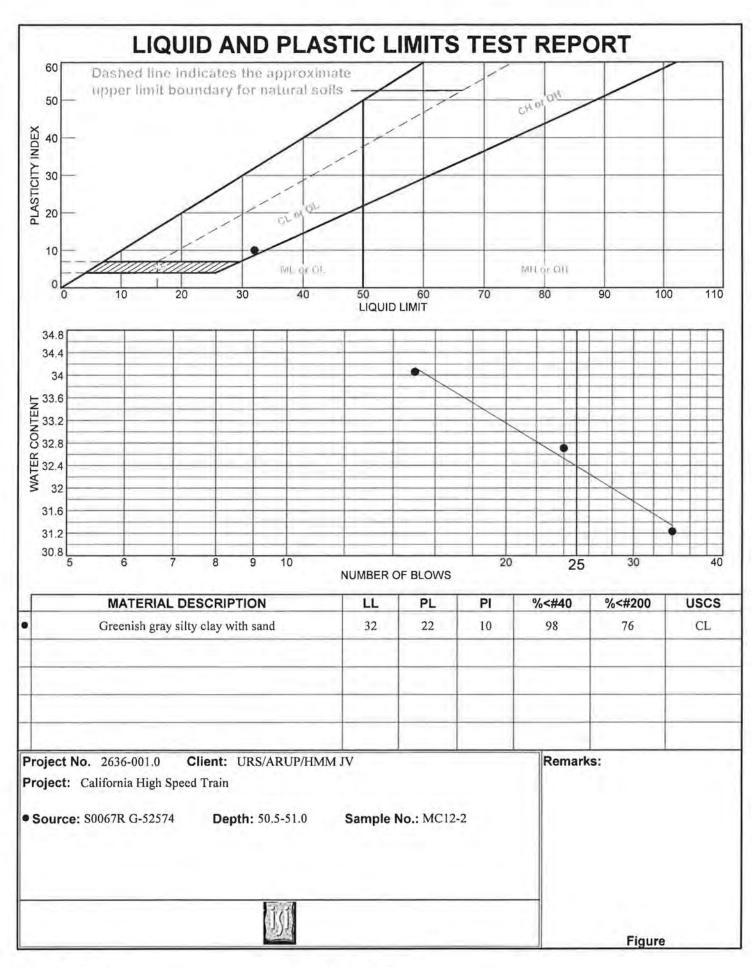
**Figure** 



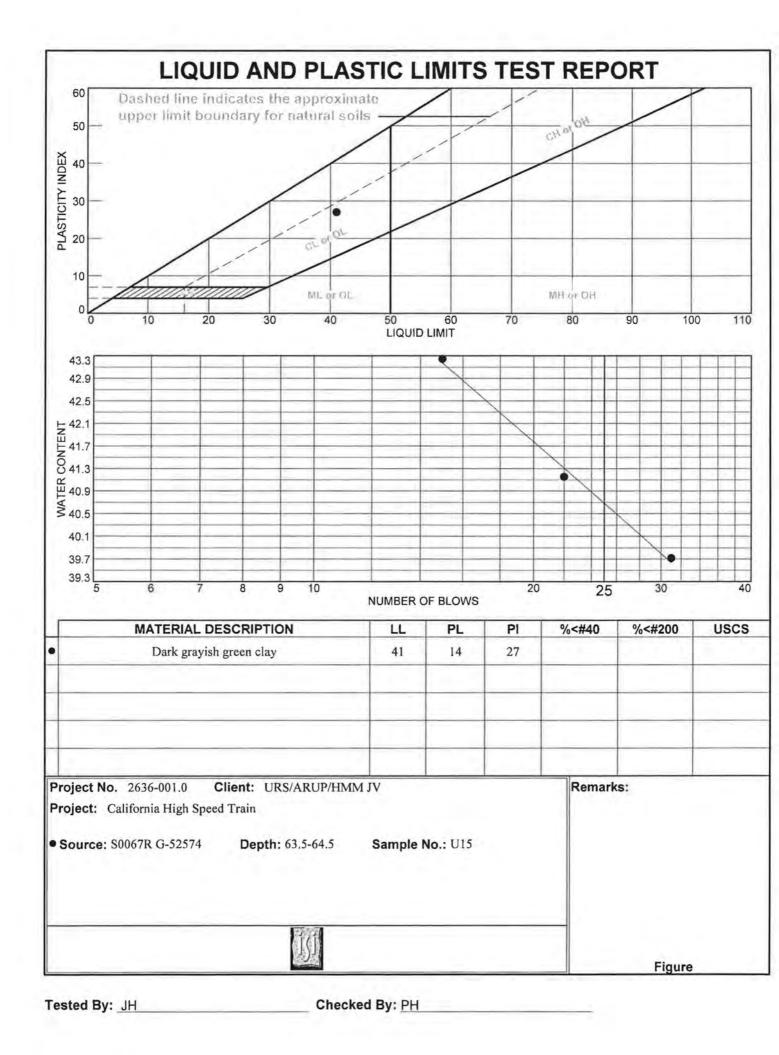


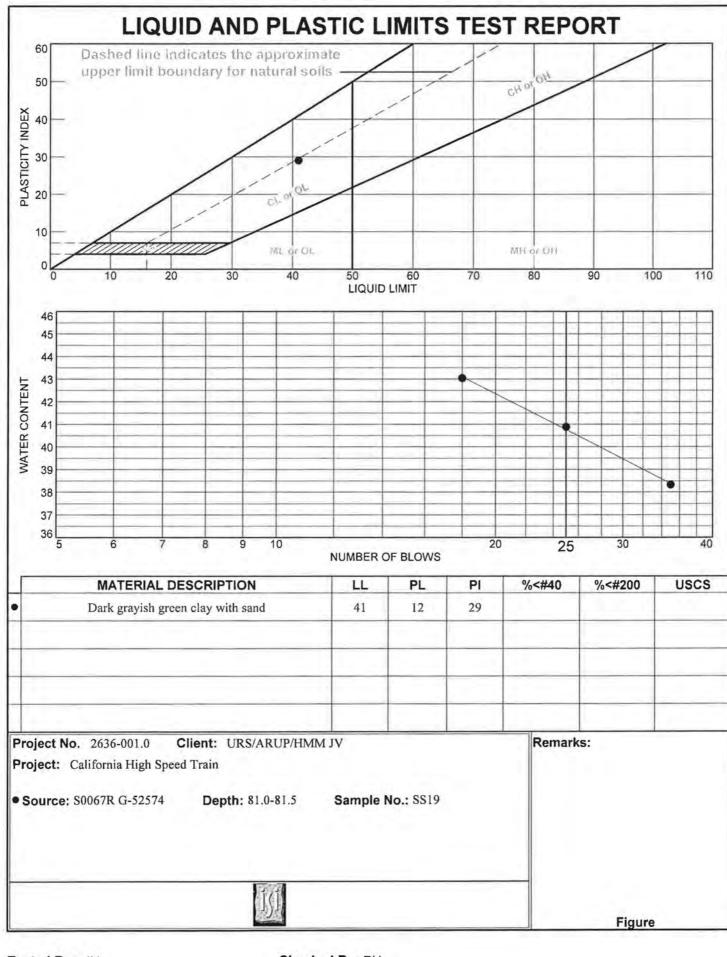
Tested By: JH

Checked By: PH



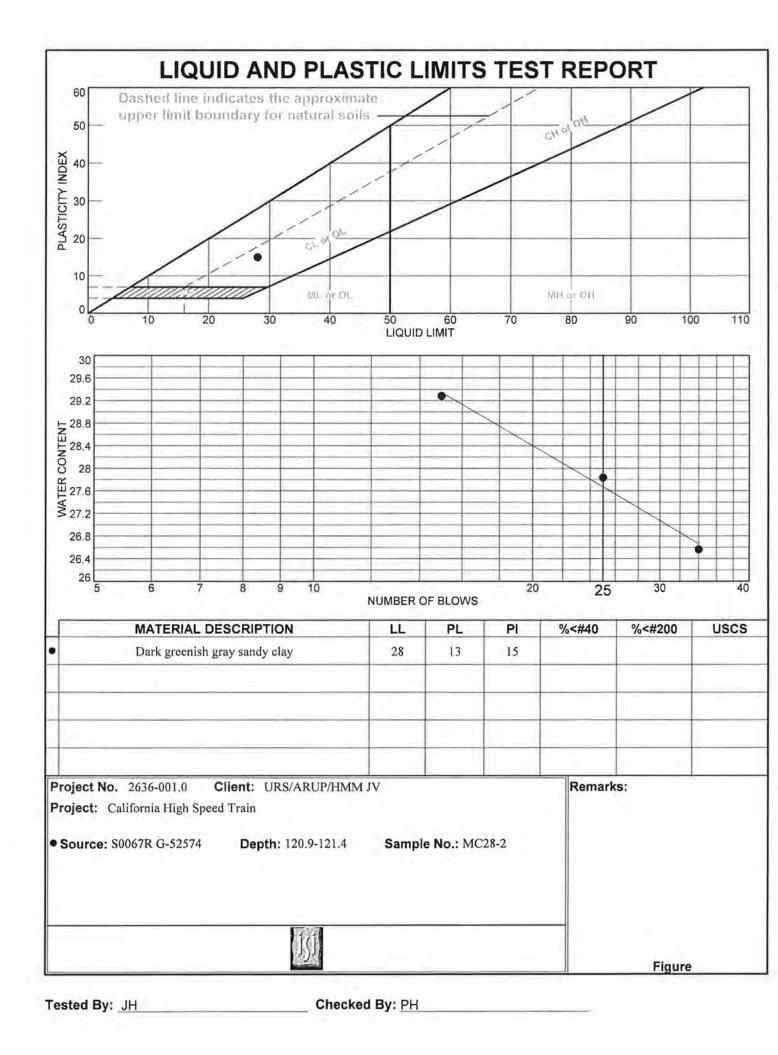
Tested By: JH Checked By: PH

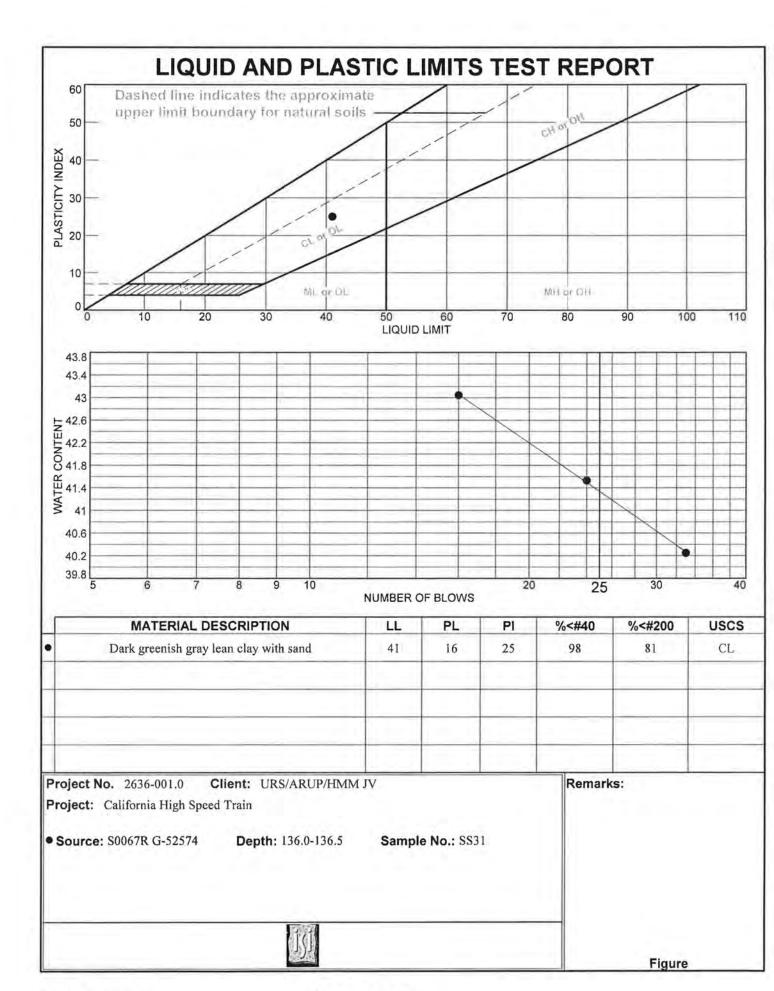




Tested By: JH

Checked By: PH





Tested By: JH Checked By: PH

#### UNCONSOLIDATED UNDRAINED COMPRESSION TEST - ASTM D2850

Client: URS/ARUP/HMM JV

Project: California High Speed Train

Job # : 2636-001.0

Boring # S0067R Sample # : MC04-1 Depth (ft) : 16

Date tested: 10/30/13

Soil: Olive brown sandy clay

Specimen:	Total wt.	=	854.5	gms	
	Ht.	=	5.440	in	
	Ave dia.	=	2.390	in	
	Area	=	4.488	sq.in	
	Volume	=	400.1	C.C.	

Volume = 400.1 c.c.

Shearing rate = 0.04 inch/min

Shearing rate = 0.75 %/min

Gs (assumed) = 2.70

Test Report:

Void ratio =	0.517	
Ht/Dia ratio =	2.28	
Moisture =	20.0	%
Total density =	133.3	pcf
Dry density =	111.1	pcf
Saturation =	104.4	%
Chamber pressure =	2880	psf
Max. deviator stress =	4089	psf
Strain @ failure =	12.33	%

	500										
Dev	1500	1	<i>f</i>								
Deviator stress (psf)	2000		1		1	-			+	+	t
ress (ps	2500	-	1	5	+	-	H		+	+	+
(Js	3000	×		1	-	H	H	+	+	+	+
	3500	-	-	1	1	-	+	+	+	-	The same of
	4000		-	t	ø	100	0	-0	Se ca	6	1

#### Data Reduction:

Dial

Read

Dial factor =	1.0	in/unit
Load factor =	1.0	lb/unit

Load

Read.

Axial

Strain

(%)

Deviator

Stress

(psf)

0.002		0.00	0.0
0.003	3.4	0.08	107.4
0.005	3.4	0.13	107.4
800.0	3.4	0.18	107.3
0.011	15.2	0.23	486.0
0.013	17.9	0.28	571.4
0.016	19.8	0.33	633.7
0.019	21.5	0.38	687.6
0.022	23.0	0.43	735.1
0.024	23.0	0.48	734.7
0.027	25.5	0.53	815.2
0.030	26.4	0.58	840.7
0.033	27.4	0.63	873.1
0.035	28.2	0.68	900.1
0.038	29.4	0.73	936.3
0.041	30.3	0.78	964.8
0.043	31.1	0.83	988.4
0.046	32.1	0.88	1020.1
0.049	33.3	0.93	1059.0
0.052	33.8	0.98	1074.2
0.054	34.9	1.03	1109.8
0.065	38.9	1.23	1234.2
0.076	42.5	1.44	1344.2
0.087	47.1	1.63	1485.7
0.098	51,1	1.84	1610.3
0.109	55.5	2.04	1745.2
0.120	59.7	2.24	1873.7
0.131	64.5	2.44	2019.0
0.142	68.7	2.64	2144.7
0.153	73.2	2.85	2280.9
0,164	77,4	3.04	2406.9
0.175	81.3	3.25	2524.4
0.186	85.8	3,45	2657.0
0.197	89.5	3.65	2768.1
0.208	92.7	3.85	2860.9
0.235	101.4	4.36	3112.5
0.262	108.9	4.85	3323.1
0.314	120.1	5.81	3629.6
0.383	128.1	7.07	3820 1
0.451	134.3	8.32	3949.7
0.519	139.4	9.57	4045.3
0.587	142.8	10.82	4086.7
0.669	145.4	12.33	4088.7
0.723	141.6	13.33	3937.7
0.778	141.0	14.33	3875.2
0.846	140.1	15.59	3794.7
0.914	139.4	16.84	3720.1
0.982	138.0	18.09	3627.6
1.050	133.7	19.34	3460.3



50067R MC04-1 16-16.5 ич 61-52574



#### UNCONSOLIDATED UNDRAINED COMPRESSION TEST - ASTM D2850

Client: URS/ARUP/HMM JV

Project : California High Speed Train

Job #: 2636-001.0

Boring # S0067R

Sample # ; U15 Depth (ft) : 63.5 Date tested : 11/04/13

Soil : Dark grayish green clay

Specimen:	Total wt.	=	1280.4	gms	
	Ht.	=	5.850	in	
	Ave dia.	=	2.850	in	
	Area	=	6.382	sqin	
	Volume	=	611.8	C.C.	

Shearing rate = 0.03 inch/min Shearing rate = 0.5 %/min Gs (assumed) = 2.70

Test Report:	Void ratio =	0.578	
	Ht/Dia ratio =	2.05	
	Moisture =	22.3	%
	Total density =	130.6	pcf
	Dry density =	106.8	pcf

Saturation = 104.1 %
Chamber pressure = 11520 psf
Max. deviator stress = 4939 psf
Strain @ failure = 4.76 %

	5000	H	3	800	9		+				1
(Jsd	4000		1			1			+		
Deviator stress (psf)	3000	1	ţ	1	l		1	+		ł	+
Deviat	2000	1			ł	1		+		ł	ł
	1000	989			1			-		1	+
	0	0	2	4	6	8	10	12	14	16	18

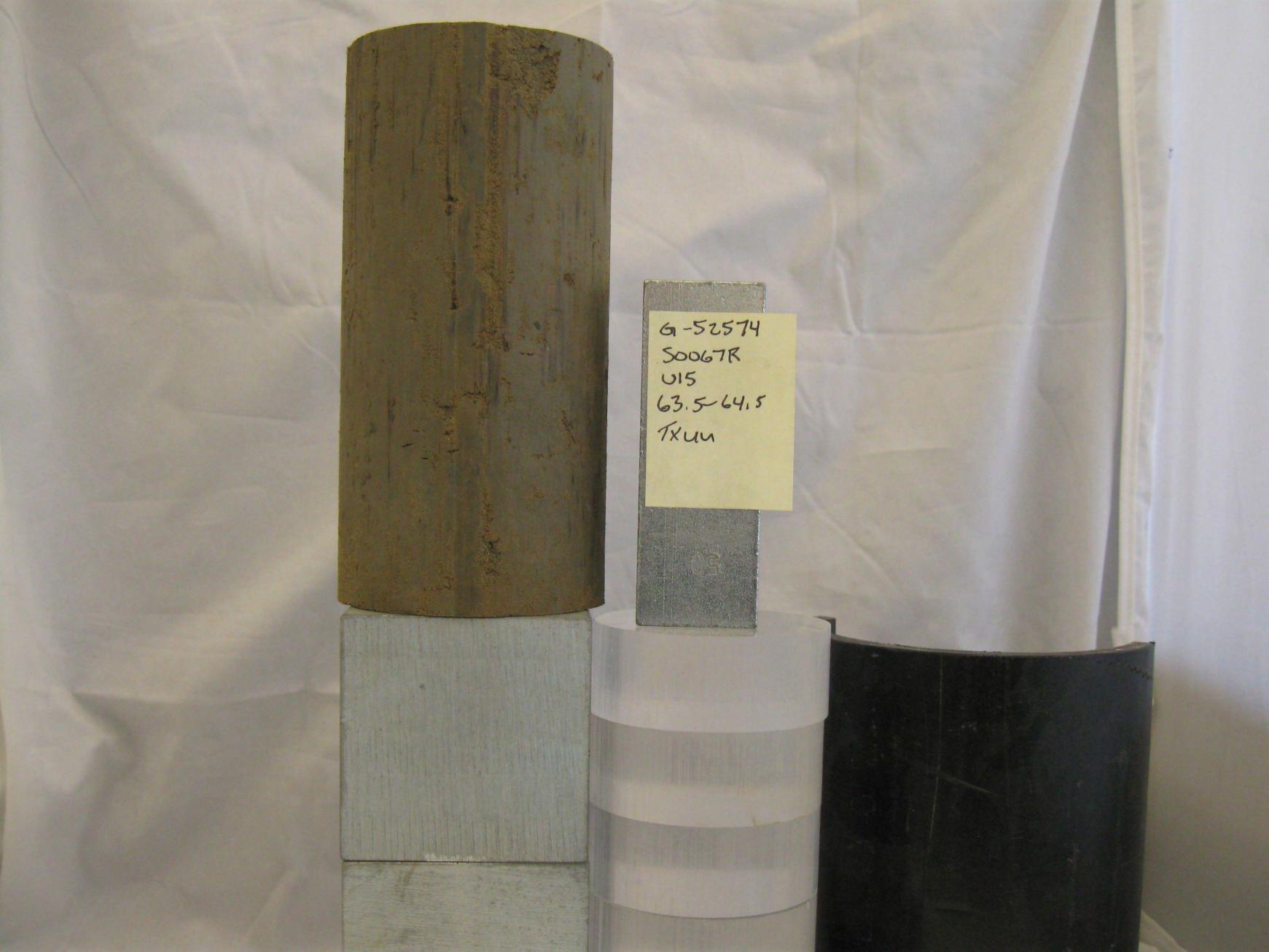
#### Data Reduction:

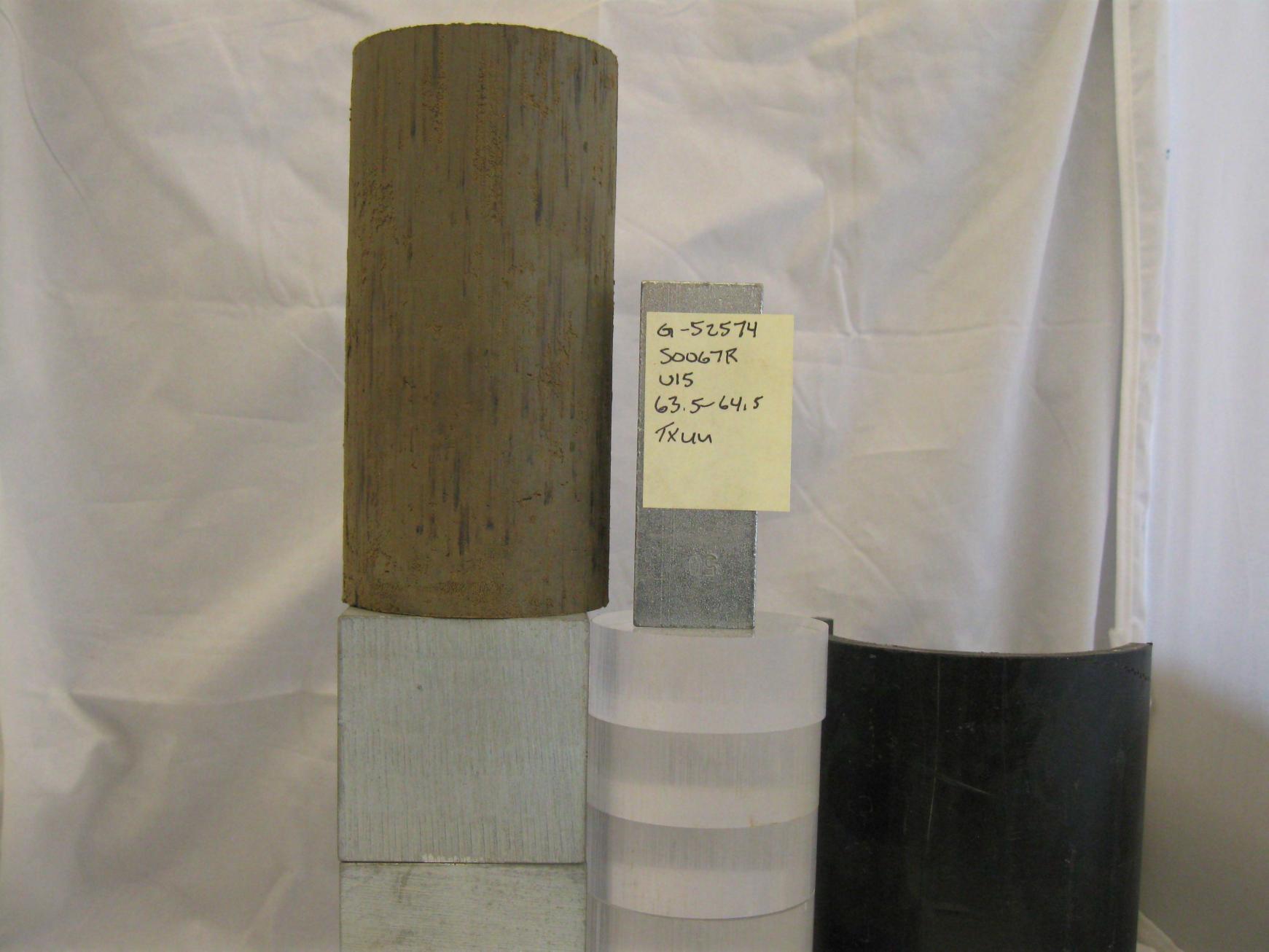
Dial factor =	1.0	in/unit
Load factor =	1.0	lb/unit

Load	Strain	Stress
Read.	(%)	(psf)
	0.00	0.0
16.3		366.9
		366.7
		366.5
		410.3
		408.8
		417.0
		497.5
		693.8
		693.5
		1113.3
		1279.9
		1426.7
		1565.0
		1684.2
		1801.4
		1909.9
		2014.1
301043		2108.2
		2211.0
		2298.8
		2522.4
		2838.2
		3136.7
		3403.3
		3636.5
		3843.1
		4017.7
		4187.1
		4314.5
		4437.4
		4544.7
		4625.4
		4711.4
		4779.8
		4898.5
		4938.7
		4892.2
		4544.8
		3775.9
133.8	9.31	2737.2
	Read.  16.3 16.3 16.3 18.2 18.2 18.5 22.1 30.9 30.9 49.6 57.0 63.6 69.8 75.2 80.5 85.3 90.0 94.3 98.9 102.9 113.1 127.5 141.2 153.5 164.4 174.1 182.4 190.4 196.6 202.7 208.0 212.1 216.5 220.1 226.8 229.8 229.6 216.1 182.0	Read. (%)  0.00 16.3 0.08 16.3 0.13 16.3 0.18 18.2 0.23 18.2 0.28 18.5 0.33 22.1 0.36 30.9 0.41 30.9 0.46 49.6 0.51 57.0 0.56 63.6 0.61 69.8 0.66 75.2 0.72 80.5 0.77 85.3 0.82 90.0 0.87 94.3 0.92 98.9 0.97 102.9 1.02 113.1 1.15 127.5 1.35 141.2 1.55 153.5 1.75 164.4 1.95 174.1 2.15 182.4 2.35 190.4 2.55 190.4 2.55 196.6 2.75 202.7 2.96 208.0 3.16 212.1 3.36 216.5 3.56 220.1 3.76 226.8 4.26 229.8 4.76 229.6 5.56 216.1 6.81 182.0 8.06

Axial

Deviator









#### UNCONSOLIDATED UNDRAINED COMPRESSION TEST - ASTM D2850

Client: URS/ARUP/HMM JV

Project: California High Speed Train

Job # : 2636-001.0 Data Reduction: Boring # S0067R

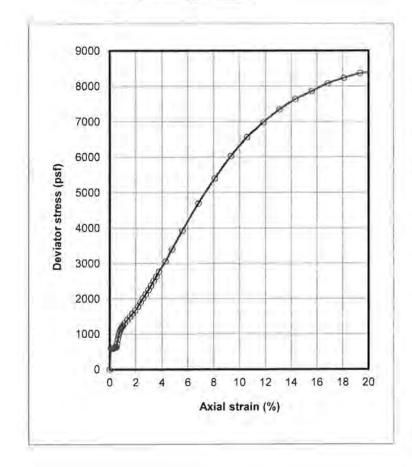
Sample # : MC30-1 Dial factor = 1.0 in/unit Depth (ft): 131 Load factor = 1.0 lb/unit

Date tested: 11/03/13

Soil ;	Dark	grayish	green	sandy	clay

Specimen:	Total wt.	=	856.9	gms	
2000	Ht.	=	5.680	in	
	Ave dia.	=	2.397	in	
	Area	=	4.513	sq.in	
	Volume	=	420.1	C.C.	
Sh	earing rate	=	0,03	inch/min	
Sh	earing rate	=	0.5	%/min	
Gs	(assumed)	=	2.70		
Test Rep	port:		Void ratio =	0.662	

Ht/Dia ratio = 2.37 Moisture = 25.5 Total density = 127.3 pcf
Dry density = 101.4 pcf
Saturation = 104.2 %
Chamber pressure = 19440 psf
Max. deviator stress = 8383 psf
Strain @ failure = 20.03 %



ricad.	11000	(10)	(60.)
-0.002		0.00	0.0
0.003	19.1	0.08	607.5
0.005	19.1	0.13	607.2
0.009	19.1	0.18	606.9
0.011	18.4	0.23	586.4
0.013	18.7	0.26	595.7
0.016	18.8	0.31	597.4
0.019	19.1	0.36	606.6
0.022	20.6	0.41	655.6
0.024	20.6	0.46	655.3
0.027	20.0	0.51	633.3
0.030	23.3	0.56	740.8
0.033	26.9	0.62	852.7
0.036	29.4	0.67	932.9
0.039	32.0	0.72	1012.4
0.042	34.0	0.77	1077.4
0.045	35.3	0.82	1118.4
0.048	36.9	0.87	1168.3
0.051	37.7	0.92	1192.7
0.053	38.8	0.97	1226.1
0.056	39.5	1.02	1246.8
0.063	41.6	1.15	1311.3
0.075	44.7	1,35	1406.7
0.086	47.4	1.55	1488.0
0.098	50.5	1.75	1581.6
0.109	53.2	1.95	1663.8
0.120	56.9	2.15	1775.7
0.132	60.9	2.35	1897.7
0.143	64.9	2.56	2017.0
0.155	68.7	2.76	2132.7
0.166	72.8	2.96	2253.8
0.178	76.7	3.16	2370.6
0.189	81.3	3.36	2506.5
0.200	85,3	3.56	2624.9
0.212	90.2	3.76	2769.3
0.240	100.1	4.27	3058.9
0,269	111.9	4.76	3399.6
0.314	130.5	5.57	3930.9
0.386	158.1	6.82	4701.8
0.456	184.1	8.07	5401.3
0.528	208.5	9.32	6033.6
0.599	230.1	10.57	6564.5
0.670	248.2	11.83	6983.7
0.741	264,8	13.08	7344.8
0.812	279.3	14.33	7635.6
0.883	291.8	15.58	7860.7
0.954	304.5	16 84	8081.0
1.026	315.1	18.09	8235.1
1.097	325.1	19.34	8366.5
1.136	328.5	20.03	8382.9

Axial

Strain

(%)

Dial

Read.

Load

Read.

Deviator

Stress

(psf)









# **Direct Shear Moisture and Density Laboratory Results**

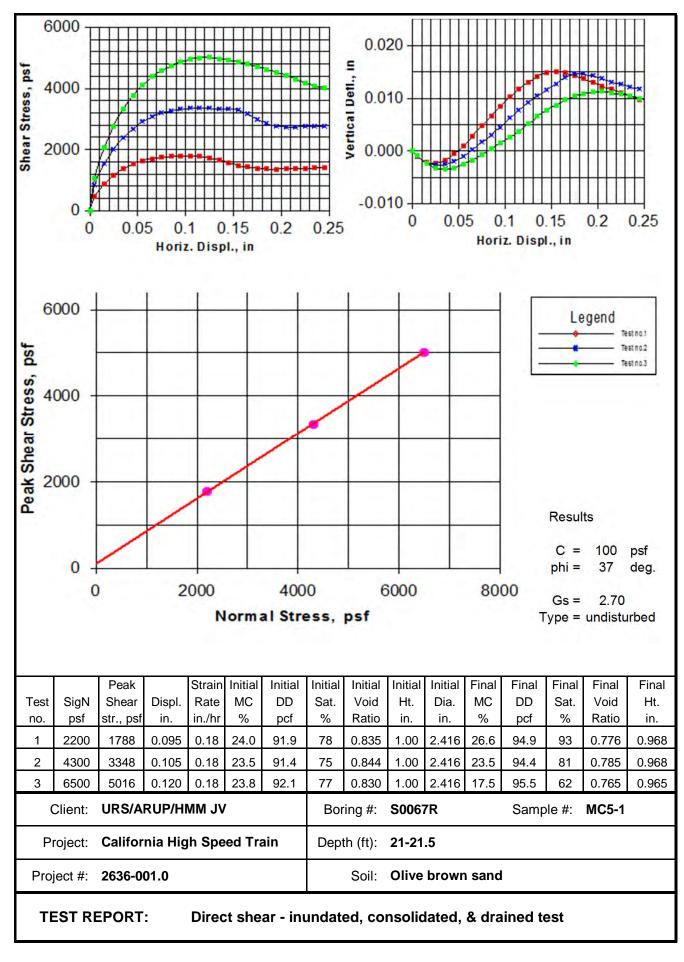
wet density (pcf) = 115.1

dry density (pcf) = 94.3

moisture (%) = 22.1

Client: URS/ARUP/HMN	I JV Bo	oring #: <b>S00</b>	67R Sample #:	MC5-1
Project: California High	Speed Train De	pth (ft): 21-2	21.5	
Project #: <b>2636-001.0</b>		Soil: Oliv	ve brown sand	

TEST REPORT: Direct shear - inundated, consolidated, & drained test



# **Direct Shear Moisture and Density Laboratory Results**

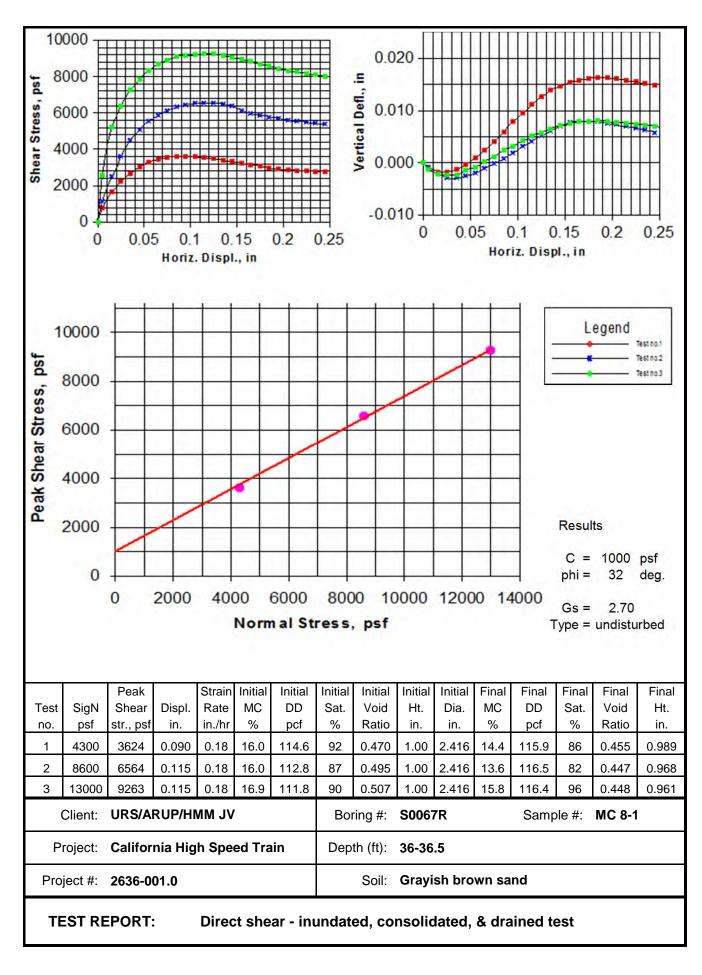
wet density (pcf) = 132.8

dry density (pcf) = 114.6

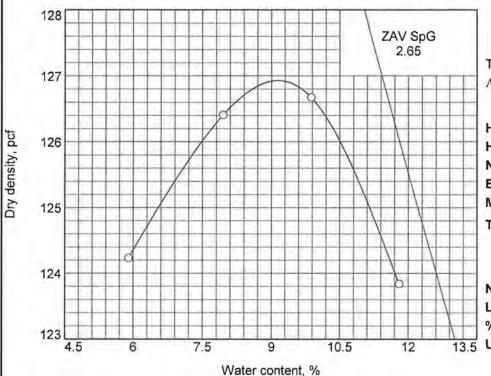
moisture (%) = 15.9

Client: URS	S/ARUP/HMM JV	Boring #:	S0067R	Sample #:	MC 8-1
Project: Calif	fornia High Speed Train	Depth (ft):	36-36.5		
Project #: <b>2636</b>	6-001.0	Soil:	Grayish brown sar	nd	

TEST REPORT: Direct shear - inundated, consolidated, & drained test



# **COMPACTION TEST REPORT**



Curve No. 52574

Test Specification:

ASTM D 1557-91 Procedure B Modified

 Hammer Wt.:
 10 lb.

 Hammer Drop:
 18 in.

 Number of Layers:
 five

 Blows per Layer:
 25

 Mold Size:
 0.03333 cu. ft.

Test Performed on Material Passing 3/8 in. Sieve

Soil Data

NM \_\_\_\_\_ Sp.G, \_\_\_\_\_

LL \_\_\_ Pl \_\_\_\_

### TESTING DATA

	1	2	3	4	5	6
WM + WS	6296.4	6337.9	6326.8	6222.1		
WM	4223.0	4223.0	4223.0	4223.0		
WW + T #1	661.7	659.9	671.9	621.3		
WD + T #1	613.0	600.6	601.0	586.7		
TARE #1	0.0	0.0	0.0	0.0		
WW + T #2						
WD + T #2						
TARE #2						
MOISTURE	7.9	9.9	11.8	5.9		
DRY DENSITY	126.4	126.7	123.8	124.2		

TEST RESULTS	Material Description
Maximum dry density = 126.9 pcf Optimum moisture = 9.1 %	Gray brown sand with clay and trace of aggregate
Project No. 2636-001.0 Client: URS/ARUP/HMM JV Project: California High Speed Train	Remarks:
O Source: S0067R G-52574 Depth: 0-5.0 Sample No.; B-01	Figure

Tested By: LL Checked By: LL/PH

### R-Value ASTM D2844 / CT301

ISI File No .: Project Name: California High Speed Train 2636-001.0 Client Name: URS/ARUP/HMM JV ISI Lab No .: G-52574

Type of Material: Gray brown sand with clay and trace of aggreg

Sampling Location: S0067R

Sample No.: B-01. 0.0 to 5.0

Test Date:	9/26/13
Run By:	LL
Checked By:	LL/PH

Specimen #		1		2		3
Compaction Pressure [psi / kPa]	325	Trees.	350	Lane	350	
Total Moisture [%]	10.7		10.1		9.7	
Density[pcf]	12	125.5		126.5		6.9
Expansion Pressure [psi / kPa]	0.00	0.00	0.00	0.00	0.00	0.00
Horizontal Pressure at 160 psi [psi / kPa]	44	303	38	262	35	241
Number of Turns D [-]	4.49		4.22		4.	20
Sample Height [in. / mm]	2.35	59.7	2.37	60.2	2.38	60.5
Exudation Pressure [psi / kPa]	240	1655	319	2200	497	3427
R-Value [-]	59	5.5	65	5.5	68	3.0
Corrected R-Value [-]	56	6.0	6	1.0	65	5.0

